

# Damage Evaluation in Dimension Limestone using Nonlinear Ultrasonics



Joshua E Love<sup>1</sup>, Megan E McGovern<sup>2</sup> and Henrique Reis<sup>1\*</sup>

<sup>1</sup>Department of Industrial and Enterprise Systems Engineering, University of Illinois, USA

<sup>2</sup>General Motors Manufacturing Systems Research Laboratory, USA

**Submission:** September 03, 2019; **Published:** September 25, 2019

**\*Corresponding author:** Henrique Reis, Department of Industrial and Enterprising Systems Engineering, University of Illinois, 104 S. Mathews Urbana, Illinois 61801, USA

## Abstract

Dolomitic limestone test samples with increasing levels of damage were obtained by exposing limestone samples to temperature levels of 100°C, 200°C, 300°C, 500°C, 600°C, and 700°C for a period of 90 minutes. The samples were then nondestructively tested using nonlinear ultrasonic in the form of a non-collinear ultrasonic wave mixing approach. In addition, the test samples' degradation of flexure strength due to damage accumulation caused by the exposure to increasing levels of temperature was also obtained using four-point bending tests. Results using the currently used non-collinear ultrasonic wave mixing approach correlate well ( $R^2 = 92\%$ ) with the corresponding obtained reduction in strength, and with results obtained using different transducer arrangements. The approach has potential applications including quantitative evaluation of damage in stone artifacts and well as to evaluate fire-induced damage in stone infrastructure.

**Keywords:** Limestone; Dimension stone; Thermal damage; Damage; Nonlinear ultrasonics; Non-collinear wave mixing; Safety

## Introduction

Limestone is frequently used as a building material due to its wide availability, comprising approximately 10% of all sedimentary rock in the world [1,2]. Typically, limestone is formed from the skeletal remains of marine organisms, such as corals, algae, and mollusks. Dolomite [ $\text{CaMg}(\text{CO}_3)_2$ ] forms as a result of the exposure of calcium carbonate to magnesium ions. Dolomitic limestone is composed of 50-90% calcite and 10-50% dolomite [1,2]. Historically, stone has been used as structural components in buildings such as the pyramids in Giza [3]. Since the 1800s, usage of stone as load-bearing structural component has largely stopped in favor of using steel frames [3-5]. Instead, stone is often used as non-load bearing, decorative cladding. Since the 1960s, 1-1/4 inch (30mm) thick panels anchored to masonry, concrete, or steel have been used as in façades for architectural applications. Since the late 1980s, stone-faced composite panels have also been used as decorative exteriors for buildings [6]. These panels typically consisted of a 3/8 in (10mm) aluminum honeycomb core, sided by a 1/16-inch (1.5mm) layer of stone (facing the outside) and by a thin lightweight reinforced polymeric composite layer such as a reinforced glass fiber composite panel. A study conducted by Chin [6] on the most common causes of failure of stone claddings determined that 40% of failures were due to a reduction in the

strength of the stone cladding due their exposure to weather, e.g., temperature variations. Additionally, 45% of failures were due to failure of the connection between the cladding and the building and 15% of failures were due to water leakage. Both connection failures [7-9] and water leakage [10] can be addressed with building regulations and codes-many of which are already in place, such as the ones supported by American Society for Testing and Materials (ASTM) standards.

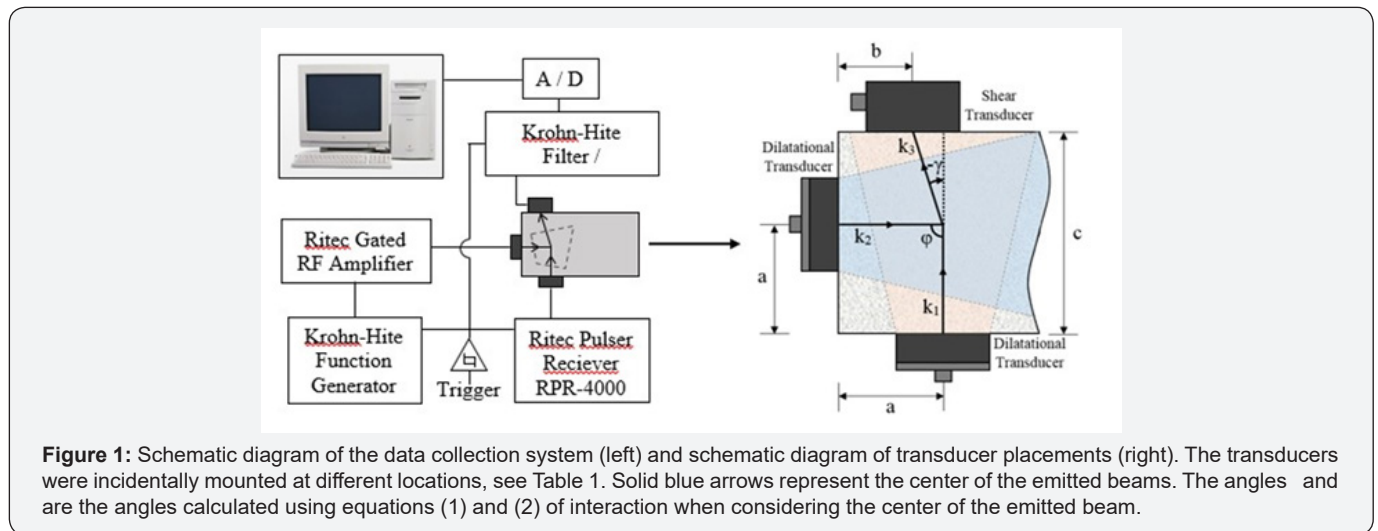
However, existing regulations and codes do not address the reduction in strength due to weathering [11-14]. Furthermore, some of the connection failures can also be attributed to loss of strength in the stone cladding surrounding the connection points [7-9, 15]. In addition, variability also exists in the quality of virgin quarried stone, differing from quarry to quarry and even between different locations in the same quarry. Unsurprisingly, when lower quality freshly quarried stone is used as cladding for buildings, it performs poorly. As an example of this is the Amoco Building in Chicago. The building was clad with 44,000 1-1/4 inch (30mm) thick Italian Carrera marble panels, which deteriorated quickly and were replaced for safety reasons with white Mt. Airy granite [16,17]. Laboratory testing determined that the marble cladding had experienced a 40% reduction in strength and an additional

30% reduction in strength was estimated over the next 10 years. In a recent study on cladding panels, Schoenberg [18] provides a review of 100 years' worth of research into stone, with a particular focus on identifying the causes of deterioration in stone paneling. Namely, panels can warp, dish, or bow, reducing the aesthetic appeal of a building and, in extreme cases, presenting a danger of falling stone. For example, marble deterioration may be caused by a variety of mechanisms. Chemical and biological attack on old buildings and monuments have been examined by several authors [19-21], and bowing has been studied for approximately 100 years [22,23]. While most bowing studies have focused on marble, examples of bowing in limestone and in granite have also been observed [24-26]. It seems that the largest factor regarding bowing in stone is thermal hysteresis in combination with moisture, due to the thermal anisotropy of calcite crystals within the stone [27].

In two previous studies by McGovern et al. [28-30], a receiving dilatational transducer was incidentally mounted on the test specimen surface to detect the generated shear horizontal nonlinear wave. The longitudinal transducer was able to detect the resultant nonlinear wave because the shear wave traveled through a highly heterogeneous media, i.e., mesoscopic media, resulting in a significant level of mode conversion and scattering. The scattering and mode conversion lead to a random spatially incoherent normal displacement at the surface, which when averaged over the large aperture of the receiving longitudinal transducer (Pan-

ametrics V1011, center frequency of 100kHz) lead to a temporally-coherently but spatially incoherent output signal. In one of these studies [28] the two primary longitudinal waves were two critically refracted longitudinal waves using dilatational transducers mounted on shear wedges. In another study [30], the two primary longitudinal waves were sent by two incidentally mounted dilatational transducers.

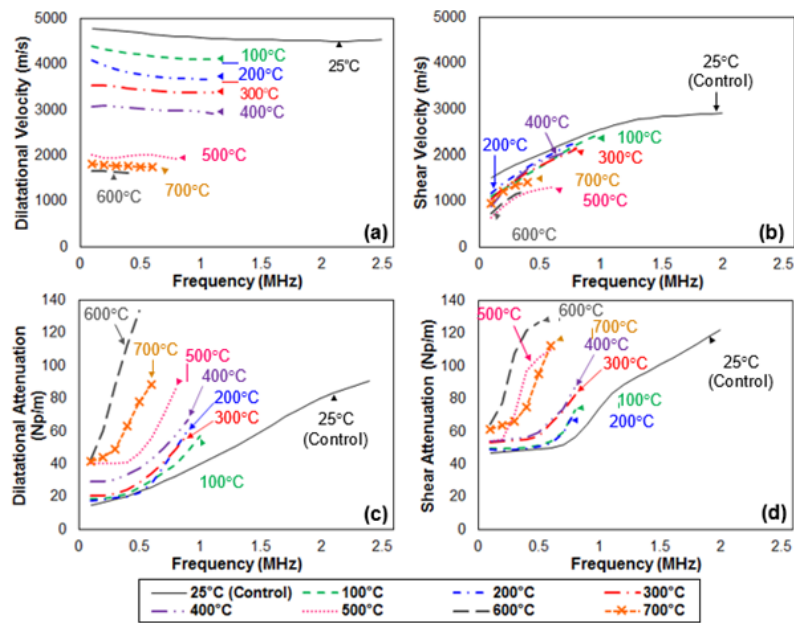
This study seeks to address whether the type of receiving transducer (shear instead of dilatational) and its placement to detect the resultant nonlinear shear wave has an impact on the observed results. In this study, the two dilatational waves are generated by two incidentally mounted dilatational transducers, and the receiving transducer is a shear transducer also incidentally mounted to the test sample. However, because the dilatational and shear velocities and corresponding attenuations depend upon the thermal damage of the specimens (see Figures 1&2), and because the overall dimension of the specimens are different from each other, the resulting shear wave meets the receiving shear transducer at a different location for each specimen, see Table 1. Please note that the overall dimension of the specimens is irrelevant provided that the relative position of the sending and receiving transducers is such that the two intercepting longitudinal waves generate the resultant shear wave, which must be captured by the receiving shear transducer.



**Figure 1:** Schematic diagram of the data collection system (left) and schematic diagram of transducer placements (right). The transducers were incidentally mounted at different locations, see Table 1. Solid blue arrows represent the center of the emitted beams. The angles  $\theta$  and  $\phi$  are the angles calculated using equations (1) and (2) of interaction when considering the center of the emitted beam.

**Table 1:** Relative position of the sending and receiving transducers with respect to the test samples.

| Specimen | a (mm) | b (mm) | c (mm) |
|----------|--------|--------|--------|
| 25°C     | 40     | 50.9   | 66.2   |
| 100°C    | 40     | 47.7   | 71.9   |
| 200°C    | 40     | 46.8   | 73.44  |
| 300°C    | 40     | 50.7   | 66.53  |
| 400°C    | 40     | 50.9   | 66.18  |
| 500°C    | 40     | 51     | 66.04  |
| 600°C    | 40     | 50.3   | 67.28  |
| 700°C    | 30     | 33     | 58.79  |



**Figure 2:** Phase velocities and corresponding attenuations: (a) dilatational and (b) shear phase velocities (m/s) as a function of frequency for the limestone specimens heated to various temperatures: (c) corresponding dilatational and (d) shear attenuations (Np/m) as a function of frequency for the limestone specimens heated to various temperatures. Figure reproduced from McGovern et al. [29].

### Nonlinear Ultrasonics

Acoustic techniques for structural health monitoring have traditionally utilized principles that are valid in the linear elastic domain. Namely, when a wave propagates in a linear elastic medium it will maintain the same frequency in the presence of flaws, regardless of whether or not the amplitude and/or phase may change in the presence of flaws. Additionally, the principle of superposition holds, where the resultant wave field associated with the two intersecting waves is the sum of the two wave fields associated with the two intersecting waves. On the other hand, if a wave propagates in a nonlinear elastic medium, the frequency will not be maintained, e.g., harmonic generation [31-34]. Furthermore, the principle of superposition does not hold true when the propagating medium is nonlinear, mainly because of the presence of higher order terms in the equations of motion [35,36]. As a result, in a nonlinear elastic medium, the intersection of two waves may result in the generation of a third wave that may have different polarization, different frequency, and may propagate in different direction. However, for a resultant nonlinear scattered wave to be generated the resonance and polarization conditions need to be satisfied [35-47].

Past experiments have observed nonlinear elastic behavior when using non-collinear wave mixing, utilizing a variety of input frequencies [35-49]. Non-collinear wave mixing has been utilized successfully to determine the higher-order elastic constants in materials [41-43] and to detect degradation such as plastic deformation and fatigue damage accumulation in metals [44,49], as well as to evaluate aging in polymers [45]. Johnson et al. [40,48] observed nonlinearly generated waves in crystalline rock and de-

veloped criteria to verify that the observed nonlinear wave was caused by the aforementioned interaction of the two primary waves within the material and not by possible instrumentation nonlinearities.

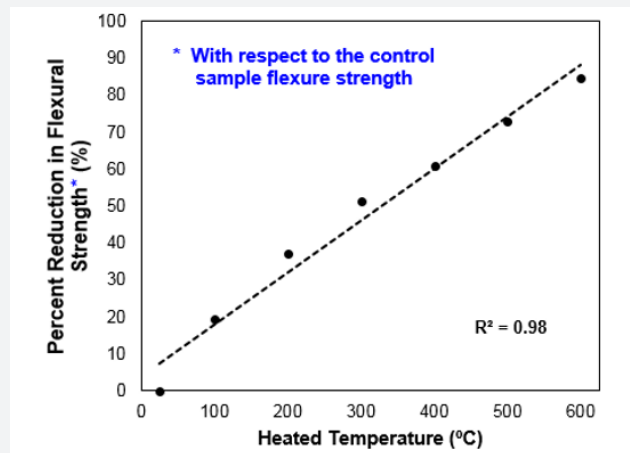
Limestone has a brick and mortar type microstructure and thus exhibits nonlinear mesoscopic elastic behavior [50,51]. In such materials, the bricks (i.e., grains, crystals, impurities) interface with each other across an elastic system, which behaves as the mortar. In limestone, the mortar is a system of asperities that holds the bricks together at the grain/crystal boundaries. Because the majority of deformation occurs within the mortar, this system of asperities is the source of most inherent nonlinear response in limestone. Additionally, limestone may be subject to various sources of damage, such as freeze-thaw cycles, acid dissolution, frost action, and salt crystallization, which cause distributed microflow populations [52]. These microflow populations make the material more susceptible to crack propagation as the microflows coalesce. Thus, the ultimate strength of the material is limited by the presence of these microflow populations, which act as nuclei when the material fractures. The presence of these microflow populations causes nonlinear distortion in propagating mechanical waves. Limestone has already been shown to exhibit nonlinear behavior by Johnson et al. [39,40,48]. In this study, test specimens with artificially induced weathering damage and different levels of damage are investigated using a non-collinear wave mixing approach by using two incidentally mounted dilatational transducers as sending transducers and using an incidentally mounted shear transducer as the receiving transducer.

### Preparation of Specimens

In McGovern et al. [28-30] samples of Illinois dolomitic limestone salvaged from windowsills were cut into blocks with nominal dimensions of 155mm x 185mm x 55mm. The heating process used and discussed by Scherer and his associates [53,54] was used to induce controlled artificial damage. Seven samples were placed in an oven, in which the temperature was increased at a rate of 50 °C per 20 minutes from room temperature (~25 °C) to each sample's individual respective desired temperature of 100 °C, 200 °C, 300 °C, 400 °C, 500 °C, 600 °C, and 700 °C. The samples were then kept at that temperature for 90 minutes. Note that 90 minutes was chosen, rather than 60 minutes used by Scherer and his

associates [53,54], to ensure uniform distribution of damage, as the specimens in this experiment were larger than the specimens used in references [53,54]. After heating, the oven was turned off and the samples were left inside to cool overnight.

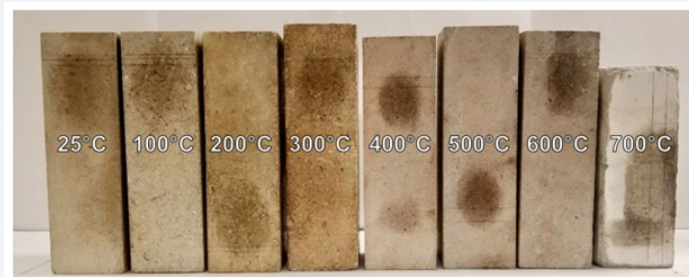
The linear acoustic properties of the test specimens, i.e., their dilatational and shear velocities and the corresponding attenuations were obtained and reported in McGovern and Reis [29]. For the benefit of the reader, Figure 2 shows the dilatation and shear velocities and corresponding attenuations as a function of damage, which was reproduced with permission from [29]. For additional discussion on specimen preparation and their linear characterization, the reader is referred to McGovern et al. [28-30].



**Figure 3:** Mean percent reduction in flexural strength (percentage) of heated samples with respect to the mean flexural strength of the control sample. The flexural strength for each specimen was averaged from five samples. Figure reproduced from McGovern et al. [29].

To evaluate the reduction in flexural strength caused by the temperature induced weathering process, five beam samples were cut from each of the specimens. The bending specimens were cut to the nominal dimensions of 180 x 55 x 15mm. These dimensions were chosen in order to satisfy the slenderness assumption of beam theory ( $h \leq 10 L$ ). Before running any flexure tests, the flexure test specimens were conditioned in an oven at 60°C for 15 hours to ensure the specimens were dry. After 13, 14, and 15 hours, the specimens were weighed to ensure their weight remained constant, implying they were completely dried. The prepared specimens were loaded under four-point bending with a supporting span of 160mm and a loading span of 40 mm,

using a displacement rate for the load head of 0.05mm/min. Load and displacement measurements were taken until the specimens fractured. The testing was conducted in accordance with ASTM standard [12]. Specimen width did not meet the standard, as dimensions were limited by original specimen geometry. Reduction in strength was measured with respect to the average strength of the control specimen in order to measure percentage reduction in strength. Figure 3 shows the average percent reduction in strength for each specimen (compared to the un-weathered specimen). For additional information regarding the bending tests, the reader is referred to Megan et al. [28-30].



**Figure 4:** Photograph of tested specimens with varying levels of damage. Note: the 700°C specimen has a different color than the other specimens due to calcination. Dark spots are due to the high vacuum grease used to affix interrogating transducers to the specimens.

The samples used in this study are the samples used in the study reported by McGovern et al. [28-30] after the five thin layers were removed from each sample to use as bending test samples. The remaining material (with nominal dimensions of 65mm x 185mm x 55mm) was used as the test specimens in this study, see Figure 4, where the darker areas are the result of the heavy vacuum grease used as couplant between the test specimens and the interrogating transducers/wedges.

### Assessment of Damage using Non-Collinear Wave Mixing

To assess damage using non-collinear wave mixing, two monochromatic dilatational waves,  $k_1$  and  $k_2$ , with frequencies  $f_1$  and  $f_2$ , respectively, intersect at an angle,  $\phi$ , to form a third, scattered shear wave,  $k_3$ , that propagates at an angle,  $\gamma$ , with respect to  $k_1$  and with frequency  $f_3 = f_1 - f_2$ , where  $f_1 > f_2$  (see McGovern and Reis [28] for a more in-depth description). Following Kobbett et al. [38], For this interaction to occur, the following equations must be satisfied,

$$\cos(\phi) = \left( \frac{c_L}{c_t} \right)^2 \left( 1 - \frac{1}{2} \frac{f_1}{f_2} \left( 1 - \frac{c_t^2}{c_L^2} \right) \left( \frac{f_2^2}{f_1^2} + 1 \right) \right) \quad (1)$$

$$\tan(\gamma) = \frac{-f_2 \sin(\phi)}{f_1 - f_2 \cos(\phi)} \quad (2)$$

where  $c_L$  and  $c_t$  are the dilatational and shear velocities respectively. Notably, there are three interdependent parameters in these equations,  $\frac{f_2}{f_1}$ ,  $\gamma$ , and  $\phi$ . Thus, if one of these parameters is chosen, the other two are also determined, see Figure 1.

In order to quantitatively assess the degree of damage in each of the specimens, this study compares the nonlinear wave generation parameter,  $\beta$ , of each specimen. To compare between specimens, the normalized nonlinear wave generation parameter,  $\beta/\beta_0$ , where  $\beta_0$  is the nonlinear wave generation parameter of the undamaged specimen, is used, as was the case in McGovern et al. [28-30]. A derivation of the normalized nonlinear wave generation parameter can be found in McGovern et al. [28-30], and is evaluated as:

$$\frac{\beta_{age}}{\beta_0} = \frac{\beta_{age}}{\beta_{25^{\circ}c}} = \frac{A_{age}^{(k_3)}}{A_{25^{\circ}c}^{(k_3)}} \quad (3)$$

$$A_{age}^{(k_3)} = \frac{A_{age}^{(k_3)}}{\exp\left(-\left[\alpha^{(k_1)} + \alpha^{(k_2)}\right]D\right) \exp\left(-\alpha^{(k_3)}D_{k_3}\right)} \quad (4)$$

where  $A_{age}^{(k_3)}$  is the recorded amplitude of the nonlinearly generated wave,  $\alpha^{(kn)}$  is the experimentally determined attenuation coefficient for the wave  $k_n$ , and  $D_{k_n}$  is the distance traversed by the wave  $k_n$  ( $D = D_{k_1} = D_{k_2}$ ). Because the nonlinear wave generation parameter,  $\beta$ , is a material property, the normalized nonlinear wave generation parameter,  $\beta/\beta_0$ , should be consistent for each specimen, regardless of the frequency ratio used. This experiment

seeks to determine whether the type of the receiving transducer has an impact on the measured normalized nonlinear wave generation parameter.

### Experimental Setup and Data Collection

Two dilatational transducers (Panametrics V413, center frequency 500kHz) and a shear transducer (Panametrics V1548, center frequency 100kHz) are incidentally mounted as illustrated in Figure 1, see Table 1.

The location of the two sending dilatational transducers is largely dictated by specimen dimensions. In this paper, the location of the receiving shear transducer is adjusted according to the specimen dimensions and is accumulated damage to assure the resultant shear wave meets the receiving shear transducer at its center.

A schematic diagram of the experimental setup is shown in Figure 1. A pulser-receiver (Ritec RPR-4000) was used to generate an eight cycles sinusoidal toneburst,  $k_1$ , at  $f_1 = 200$ kHz. A function generator (Krohn-Hite Model 5920) was used to generate an eight cycles sinusoidal toneburst,  $k_2$ , which was amplified by a gated amplifier (Ritec GA-2500A). This signal was iteratively swept from  $f_2 = 60$ kHz to  $f_2 = 180$ kHz in 2kHz increments ( $\frac{f_2}{f_1} = 0.3$  to 0.9). The sample rate was 50MHz to avoid trigger jitter [39,48]. For each iteration, data was averaged 350 times to mitigate the effects of noise and scatter. The received wave was filtered and amplified by a 4-Pole Butterworth filter (Krohn-Hite Model 3945), and then sent to the computer. To measure the nonlinearly scattered wave, three measurements were taken:

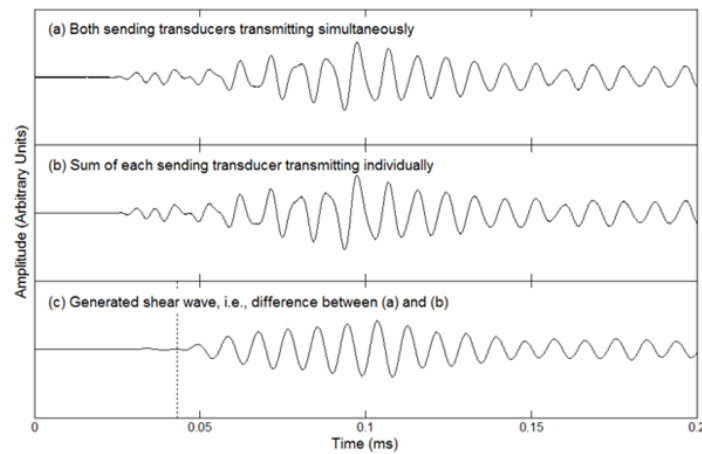
- A. A measurement with both longitudinal sending transducers transmitting.
- B. A measurement with only one of the sending transducers transmitting.
- C. A measurement with only the other sending transducer transmitting.

By subtracting, the two individual measurements from the measurement obtained when the two sending transducers operate simultaneously, the nonlinearly scattered wave (i.e., the difference signal) can be isolated. It should be noted that this is not a perfect subtraction to isolate the nonlinear scattered wave, as some of the energy from the primary waves is used to generate the nonlinear scattered wave. Therefore, the amplitude of both transducers operating simultaneously should be slightly lower than the sum of the amplitudes of the transducers operating individually. As a result, the difference signal, i.e., the nonlinear scattered wave includes a portion of the amplitude of the primary waves. To account for this, the resultant measurement was filtered around the theoretically predicted frequency,  $f_3$ .

The location of the dilatational sending transducers was largely determined by specimen geometry. The location of the shear receiving transducer was determined by utilizing equation

(2). For the other specimens, it was assumed that utilizing the same calculated angle,  $\gamma$ , would be an appropriate location, similar to the experimental method in McGovern and Reis [28]. The exact configuration of the transducers for each specimen along with specimen dimensions can be seen in Table 1. Additionally, for each independent test, all transducers were removed from the test sample, residual couplant was cleaned from the surface of the sample and the transducers, new couplant was applied to the transducers, and the transducers were affixed to the sample.

Figure 5 shows the received signals in the time domain for the un-weathered 25°C specimen. Figure 5a shows the signal recorded when both sending transducers are operating simultaneously, and Figure 5b shows the sum of the signals recorded when each sending transducer operates individually. Figure 5c shows the scattered shear wave generated by the nonlinear interaction when both sending transducers operate simultaneously. The observed time-of-arrival differed from the predicted time-of-arrival within an error of 5%. In Figure 5, the amplitude of the generated shear wave is scaled up 10 times.

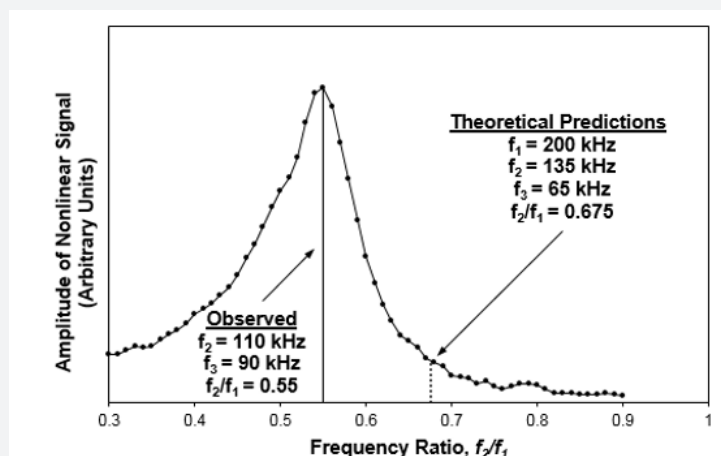


**Figure 5:** Received signals in the time domain: (a) signal recorded when both sending transducers are operating simultaneously; (b) sum of the signals recorded when each sending transducer operates individually; (c) the shear wave generated by the nonlinear interaction when both sending transducers operate simultaneously. The observed time of arrival differed from the predicted time of arrival within an error of 5%. The amplitude of the generated shear wave is scaled up 10 times. The weathered 100°C specimen was used to obtain this data.

### Experimental Results

When analyzing the collected data, the Johnson et al. [39] selection criteria was used to ensure the observed signal was in fact the nonlinearly generated scattered shear wave and not an artefact of equipment nonlinearities such as nonlinearities in the used couplant, amplifiers etc. Namely, the amplitude criteria was met by varying the voltage of the primary waves and noting that the

received scattered wave amplitude changed in a manner proportional to the primary scattered wave amplitudes. The directionality criteria was also met by the nature of the experiment, i.e. placement of the sending and receiving transducers. The time-of-flight criteria was met by observing that the experimental arrival times were consistent with the expected arrival times for the nonlinearly generated signal.



**Figure 6:** Sample amplitude distribution as the frequency ratio was changed for the 100°C specimen. The discrepancy between theoretical and observed peak frequency ratios are attributed to the mesoscopic behavior of limestone, deviations from the theoretical assumptions used in the non-collinear wave mixing formulation [40,48] and the transducer placement used in this study.

To identify the frequency ratio that produces the maximum amplitude of the nonlinearly generated shear wave, one transducer was set to send a 200kHz signal for each test, whereas the signal for the other transducer was swept from 60kHz to 180kHz in 2kHz intervals. A sample of experimental results for one frequency sweep can be seen in Figure 6. Figure 6 shows the frequency ratio that experimentally corresponds to the observed maximum amplitude as compared with the theoretical predicted frequency ratio. The discrepancy between theoretical and observed peak frequency ratios are attributed to the mesoscopic behavior of limestone, deviations from the theoretical assumptions used in

the non-collinear wave mixing formulation [38], and the transducer placement used in this study.

Please note that under the assumptions associated with to the traditional non-collinear wave mixing formulation [38], i.e., plane longitudinal primary waves and homogeneous isotropic material, the nonlinear wave shown in Figure 5 would have zero magnitude. However, mainly because of the beam spread of the two primary waves and the mesoscopic behavior of the material, the resultant nonlinear wave has a finite amplitude, as it is illustrated in Figure 5.

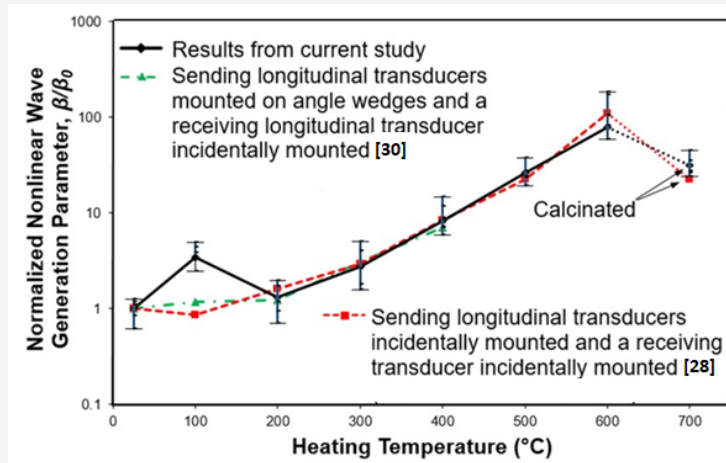


Figure 7: Normalized nonlinear wave generation parameter as a function of heating temperature. The results from this experiment (solid black) are presented alongside data reproduced from McGovern et. al [28,30].

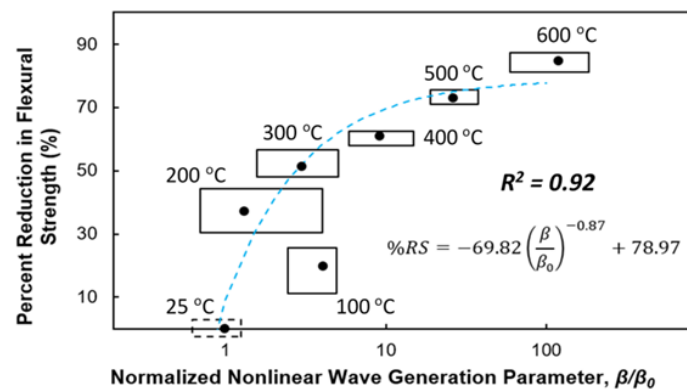


Figure 8: Percent reduction in flexural strength (with respect to the mean strength of the control sample) as a function of the normalized nonlinear wave generation parameter. A power fit is applied to the data without considering the 100°C sample, as it is believed damage was not uniform in the sample ( $R^2 = 0.92$ ). The error box of the 25°C specimen drops below zero percent reduction in strength due to the normalization process. Considering the data point corresponding to the 100°C sample leads to a correlation coefficient of  $R^2 = 0.77$ . The data point corresponding to the 700°C specimen is not included because the materials is not limestone due to the calcination transformation that takes place at these temperatures.

Figure 7 shows a comparison of the results obtained in this experiment with various other transducer arrangements used in past experiments by McGovern et al. [28-30]. Notably, there is a discrepancy between the normalized nonlinear parameter for the 100 °C specimen between this experiment and past experiments. This deviation is explained by the presence of residue in

the 100 °C specimen left during the cutting process using oil to cool the cutting saw. This residue was burned off for higher temperature specimens, but for the 100 °C specimen the temperature was not sufficiently high. In addition, the damage distribution for the 100 °C specimen was not uniform throughout its volume because of the lower temperature. Apart from the 100 °C specimen,

observed normalized nonlinear wave parameter values match closely to previous studies utilizing dilatational receiving transducers, which validates previous experiments and implies shear transducers and dilatational transducers can both be used as receiving transducers to determine the normalized nonlinear wave generation parameter, see McGovern et al. [28,30]. In Figure 7, as it was also observed in previous experiment [28-30], the normalized nonlinear parameter changes drastically from the 600 °C specimen to the 700 °C specimen. This is due to the calcination transformation that takes place at these temperatures. For additional information regarding the calcination decomposition, the readers are referred to [28-30, 55,56].

Figure 8 shows a correlation between the normalized nonlinear wave generation parameter and the reduction in flexural strength ( $R^2 = 0.92$ ). The Figure also shows that the percent reduction in flexural strength error box is negative for the control specimen. This is an artefact of the normalization process, as percent reduction in flexural strength is measured with respect to the mean flexural strength of the control specimen. Apart from the results of the 100 °C specimen, the power fit seems to accurately relate the normalized nonlinear parameter with the reduction in flexural strength. The data point corresponding to the specimen damaged using the 100°C is not included because of the lack of uniformly distributed damage in the 100°C specimen; because the specimens were cut using oil as the saw lubricant, and the 100°C was not sufficiently high to eliminate/reduce the oil residue in the middle of the specimen.

Finally, results of the receiving transducer placement shown in Table 1 also indicate the maximum deviation from the position selected for the undamaged specimen is about 15mm for the 600 °C specimen. Note that the 700°C specimen is not considered mainly because is a much thinner specimen, i.e., it has a lower value of the dimension  $c = 33\text{mm}$ , as compared to the other specimens, see Table 1. Considering that the diameter of the receiving shear transducer (Panametrics V1548, center frequency 100kHz) is 38mm (1.5in), a conclusion can be reached that one can place the receiving transducer assuming the material properties of the undamaged specimen without altering the results, as it was done in previous studies by McGovern and Reis [28-30].

## Conclusion

Dolomitic limestone specimens were artificially weathered at temperatures of 25°C, 100°C, 200°C, 300°C, 400°C, 500°C, 600°C, and 700°C. Non-collinear wave mixing of two dilatational waves was used to generate a scattered shear wave. This interaction of the two intercepting longitudinal waves was used to characterize the limestone specimens of varying degrees of thermal damage. While the resultant scattered shear wave was captured using a shear transducer mounted on the test specimens' surface, its location varied from specimen to specimen because the velocities of each specimen depend of its damage accumulation. This study shows that the use of both dilatational and shear transducers as

receiving transducers is acceptable, i.e., both lead to relatively similar normalized nonlinear wave generation parameter values,  $\beta/\beta_0$ . It was also observed that placing the receiving transducers at different locations did not significantly affect the results mainly because of the relatively large diameter of the receiving sensor, which validates the simpler approaches used by McGovern et al. [28,30], where the relative location of the transducers' placement is constant. This approach has the potential to quantitatively evaluate damage in monuments and art objects for the purpose of taking remedial actions as discussed by Scherer and his associates [53,54], as well as to evaluate fire induced damage in stone such as in tunnels.

## References

1. Kuzvart M (1984) *Industrial Minerals and Rocks*, 1<sup>st</sup> (ed) Elsevier, New York.
2. Freas RC, Hayden JS, Pryor CA (2006) *Limestone and Dolomite Industrial Minerals and Rocks: Commodities, Markets and Uses*. In: Kogel JE, Trivedi NC, Barker JM, Krukowski ST (Eds.), Colorado: Society for Mining, Metallurgy, and Exploration. 7<sup>th</sup> (Ed), Pp: 581-597.
3. Fletcher B (1975) *A History of Architecture*, University of London, Athone Press, Pp: 213-219.
4. Ashurst J, FG(Eds.) (2000) *Conservation of Building and Decorative Stone*. 2<sup>nd</sup> (Eds) John Butterworth-Heinemann, London, England, p: 1-19.
5. Scheffler MJ, DS Kneezel, DS (2007) "Testing of Composite Stone Faced Aluminum Honeycomb Panels, In: KR Hoigard, Scheffler, MJ (Eds.) *Dimension Stone Cladding: Design, Construction, Evaluation, and Repair*, ASTM STP 1499, American Society for Testing and Materials, West Conshohocken, PA, pp. 11-24.
6. Chin IR (2000) *Common Causes of Failures of Stone Claddings on Buildings*. In: Hoigard KR, (Ed.) *Dimension Stone Cladding: Design, Construction, Evaluation, and Repair*, ASTM STP 1394, American Society for Testing and Materials, West Conshohocken, PA, pp. 151-160.
7. Lammert BT, Hoigard KR (2007) *Material Strength Considerations in Dimension Stone Anchorage Design*. In: K.R. Hoigard, and M.J. Scheffler, (Eds.) *Dimension Stone Cladding: Design, Construction, Evaluation, and Repair*, ASTM 1499, American Society for Testing and Materials, West Conshohocken, PA, pp. 40-57.
8. ASTM Standard C 1354 (2001) *Standard Test Method for Strength of Individual Stone Anchorages in Dimension Stone*. Annual Book of Standards 4(7), ASTM International, West Conshohocken, PA.
9. Conroy K, Hoigard kR (2007) *Stiffness Considerations in Dimension Stone Anchorage Design*. In: K.R. Hoigard, and M.J. Scheffler, Eds. *Dimension Stone Cladding: Design, Construction, Evaluation, and Repair*, ASTM STP 1499, American Society for Testing and Materials, West Conshohocken, PA, pp. 58-68.
10. ASTM Standard E 2128-01 (2001) *Standard Guide for Evaluating Water Leakage of Building Walls*. Annual Book of Standards, ASTM International, West Conshohocken, PA.
11. Naggatz SG, Gerns EA (2007) *Full-Scale Flexural Strength Testing for Stone Cladding Design*. In: Hoigard KR, Scheffler MJ (Eds.) *Dimension Stone Cladding: Design, Construction, Evaluation, and Repair*, ASTM STP 1499, American Society for Testing and Materials, West Conshohocken, PA, pp. 3-10.



12. ASTM Standard C 880-06 (2007) Standard Test Method for Flexure Strength of Dimension Stone, Annual Book of Standards 4(7), ASTM International, West Conshohocken, PA.
13. ASTM Standard C 170 (2001) Standard Test Method for Compressive Strength in Natural Building Stone. Annual Book of Standards 15(6) ASTM International, West Conshohocken, PA.
14. ASTM Standard C 99 (2000) Standard Test Method for Modulus of Rupture in Natural Building Stone. Annual Book of Standards, V15.06, ASTM International, West Conshohocken, PA.
15. ASTM Standard C 1201 (2001) Standard Test Method for Structural Performance of Exterior Dimension Stone Cladding Systems by Uniform Static Air Pressure Difference. Annual Book of Standards 4(7), ASTM International, West Conshohocken, PA.
16. Unknown (1995) Proceedings of Seminar on Recladding of the Amoco Building in Chicago, Illinois, Chicago Committee on High Rise Buildings, Chicago, Illinois, Report No. 15, p. 3-5.
17. Cohen, J.M., and Monteiro, P.J.M. (1991) Durability and Integrity of Marble Claddings: A State-of-the-Art Review. Journal of Performance of Constructed Facilities, American Society of Civil Engineers, (ASCE), 5: 113-124.
18. Schouenborg B, Grellk B, Malage K (2007) Testing and Assessment of Marble and Limestone (TEAM)- Important Results from a Large European research Project on Cladding Panels. In: Hoigard KR, Scheffler MJ, (Eds.) Dimension Stone Cladding: Design, Construction, Evaluation, and Repair, ASTM STP 1499, American Society for Testing and Materials, West Conshohocken, PA, pp. 124-137.
19. Feddema JJ, Meierding TC (1987) Marble Weathering and Air Pollution in Philadelphia. Atmos. Environ., 21(1): 143-157.
20. Cooke RU (1995) Using Gravestones to Assess Changing Rates of Weathering in the United Kingdom, Earth Surf. Processes Landforms, 20:531-54.
21. Srinivas SY (1996) Weathering Rates of Marble in Laboratory and Outdoor Conditions. J of Environment Engineering, 22(9): 856-863.
22. Kessler DW (1919) Physical and Chemical Tests on the Commercial Marbles of the United States, Technological Papers of the Bureau of Standards, No. 123, National Institute of Standards and Technology, Gaithersburg, MD.
23. Grimm WD (1999) Observations of Reflections on the Deformation of Marble Objects Cause by Structural Breaking Up, Zeitschrift der Deutschen Geologischen Gesellschaft (J of German Geological Society), Band 150, Teil 2, Themenheft: Marmor-Konservierung, Stuttgart 150(2):195-235.
24. Winkler EM (1999) Buttress Expansion in Granit Rocks in the Field and on Buildings. Proceedings of the 4<sup>th</sup> Internationales Kolloquium Werkstoffwissenschaften und Bauinstandsetzen, Technical Academy of Esslingen, Fraunhofer IRB Verlag, pp. 537-542.
25. Mauco A, Mittic B, Maldenovic A, Grellk B (2006) Deterioration of the Granodiorite Natural Stone Façade- Case Example Maximarket, Ljubljana, RMZ-Materials and Geoenvironment, 53(1), 23-37.
26. TEAM, Testing and Assessment of Marble and Limestone (2005), Final Technical Report. EC-Project: TEAM- G5RD-CT-2000-00233.
27. Siegesmund S, Ullemeyer K, Weiss T, Tschegg EK (2000) Physical weathering of marbles caused by anisotropic thermal expansion. International Journal of Earth Science, 89(1): 170-182.
28. Megan E, Govern Mc, Reis H (2015) Damage characterization in dimension limestone cladding using non- collinear ultrasonic wave mixing. Optical Engineering, 55(1): 011012-12.
29. Megan E, Govern Mc, Reis H (2014) Linear and nonlinear characterization of limestone rock using a non- collinear ultrasonic wave mixing. Proceedings of SPIE 9064, Health Monitoring of Structural and Biological Systems, 906404.
30. Megan E, Govern Mc, Reis H (2017) Nonlinear ultrasonic damage characterization in limestone. Research in Nondestructive Evaluation 28(4): 226-240.
31. Cantrel JH, Salama H (1991) Acoustoelastic Characterization of Materials. International Materials Review 36: 125-145.
32. Cantrel JH., Yost WT (2001) Nonlinear Ultrasonic Characterization of fatigue microstructures. International Journal of Fatigue 23: 487-490.
33. Kim J, Jacobs J, Qu J, Littles, JW (2006) Experimental characterization of fatigue damage in nickel- base super alloy using nonlinear ultrasonic waves. J of Acous. Society of America, 120: 1266-1273.
34. Pruell C, Kim J, Qu J, Jacobs LJ (2007) Evaluation of Plasticity Driven Material Damage using Lamb Waves. Applied Physics Letters, 91, 231911.
35. Murnaghan FD (1951) Finite Deformation of an Elastic Solid, New York: John Wiley & Sons, Inc.
36. Landau LD, Lifshitz EM (1970) Theory of Elasticity, 2<sup>nd</sup> (Ed). New York: Pergamon Press.
37. Gol'dberg ZA, (1960) Interaction of Plane Longitudinal and Transverse Elastic Waves. Soviet Phys. Acoust., 6(3): 306-310.
38. Jones GL, Kobett DR (1963) Interaction of Elastic Waves in an Isotropic Solid. Journal of Acoustical Society of America, 35(1): 5-10.
39. Johnson PA, Zinszner B, Rasolofosaon PNJ (1996) Resonance and Elastic Nonlinear Phenomena in Rock. Journal of Geophysical Research 101(B5):11553-11564.
40. Johnson PA, Shankland TJ, O'Connell RJ, Albright JN (1987) Nonlinear Generation of Elastic Waves in Crystalline Rock. Journal of Geophysical Research 92(B5): 3597-3602.
41. Rollins FR, Taylor LH, Todd PH (1964) "Ultrasonic Study of Three-Phonon Interactions. II. Experimental Results," Physical Review 136(3A): A597-A601.
42. Taylor LH, Rollins FR (1964) "Ultrasonic Study of Three-Phonon Interactions. I. Theory," Physical Review 136(3A): A591-A596.
43. Rollins FR (1965) "Phonon Interactions and Ultrasonic Frequencies," Proceedings of the IEEE 53(10): 1534-1539.
44. Croxford AJ, Wilcox PD, Drinkwater BW, Nagy PB (2009) The Use of Non-Collinear Mixing for Nonlinear Ultrasonic Detection of Plasticity and Fatigue. Journal of Acoustical Society of America, 126(5): EL117-EL122.
45. Demcenko A, Akkerman R, Nagy PB, Loendersloot R (2012) Non-Collinear Wave Mixing for Nonlinear Ultrasonic Detection of Physical Aging in PVC. NDT&E International 49:34-39.
46. Dunham RW, Huntington HB (1970) Ultrasonic Beam Mixing as a Measure of the Nonlinear Parameters of Fused Silica and Single-Crystal NaCl. Physical Review 2(4):1098-1107.
47. Zarembo KL, Krasil'nikov VA (1970) Nonlinear Phenomena in the Propagation of Elastic Waves in Solids. Soviet Physics 13(6): 778-797.
48. Johnson PA, Shankland TJ (1989) Nonlinear Generation of Elastic Waves in Granite and Sandstone: Continuous Wave and Travel Time Observations. Journal of Geophysical Research 94(B12):17729-17733.
49. Nagy PB, (1998) Fatigue Damage Assessment by Nonlinear Ultrasonic Materials Characterization. Ultrasonics 36: 375-381.
50. Guyer RA, Johnson PA (1999) Nonlinear Mesoscopic Elasticity: Evidence for a New Class of Materials. American Institute of Physics, Physics Today 52(4): 30-36.

51. Guyer RA, Johnson PA (2009) Nonlinear Mesoscopic Elasticity: The Complex Behavior of Granular Media Including Rocks and Soil, Weinheim: Wiley.
52. Miglio BF, Richardson DM, Yates TS, West D (2000) Assessment of the Durability of Porous Limestones: Specification and Interpretation of Test Data in UK Practice, Dimension Stone Cladding: Design, Construction, Evaluation, and Repair, In: Hoigard KR, (Ed.) ASTM STP 1394, , ASTM, W. Conshohocken, Pennsylvania.
53. Sassoni E, Naidu S, Scherer GW (2011) The use of hydroxyapatite as a new inorganic consolidant for damaged carbonated stones, Journal of Culture Heritage 12: 346-355.
54. Franzoni E, Sassoni E, Scherer W, George, NaiduS, (2013) Artificial weathering of stone by heating. Journal of Culture Heritage 14S: e85-e93.
55. Britton HTS, Greg SJ, Windsor GW (1952) The calcination of dolomite: part II. The thermal decomposition of Dolomite. Transactions of the Faraday Society 48:70-75.
56. Kumar GS, Ramakrishnan A, Hung, YT (2017) Lime Calcination In: Wang LK, Hung YT, Shamma NK (Eds.) Advanced Physicochemical Treatment Technologies Handbook of Environmental Engineering, Hamana Press, New Jersey 5: 611-633.



This work is licensed under Creative Commons Attribution 4.0 License  
DOI: [10.19080/CERJ.2019.09.555755](https://doi.org/10.19080/CERJ.2019.09.555755)

### Your next submission with Juniper Publishers will reach you the below assets

- Quality Editorial service
- Swift Peer Review
- Reprints availability
- E-prints Service
- Manuscript Podcast for convenient understanding
- Global attainment for your research
- Manuscript accessibility in different formats  
( Pdf, E-pub, Full Text, Audio)
- Unceasing customer service

Track the below URL for one-step submission  
<https://juniperpublishers.com/online-submission.php>